



**BID NO. 26-004
PUBLIC WORKS STEEL COLUMNS REPAIR**

ADDENDUM NO. 1

January 16, 2026

This addendum shall modify and become a part of the bid document. Specifications not specifically mentioned in this Addendum are not altered by any changes, amendments, deletions, or additions.

Reply to Requests for Information:

1. Could you provide the height of the beams to be repaired for project 26-004.

The height of the columns and any other existing conditions are for the contractor to verify in the field during one of the mandatory pre-bid site visits

2. Could we please get a set of the as-built structural drawings.

A complete set of as-built drawings is not available; bidders are to verify as-built conditions in the field during one of the mandatory pre-bid site visits. Attached are some record structural and architectural drawings of the building for information only; do not solely rely on these sheets for any take offs or other purpose; as it may not be an accurate representation of existing conditions.

3. Will AMPP QP1/QP2 be required for blasting and painting of the steel?

No.

The proposal submission deadline is Friday, January 30, 2026, at 3:00 PM. Any proposal response received after the designated closing time will be returned unopened.

Any questions concerning this Addendum should be addressed to Monica Powery, Director of Purchasing at (561) 642-2039.

All Proposers shall acknowledge receipt of this addendum by annotating the proposal signature page with the addendum number, completing the section below, and returning with the proposal response to verify receipt.

Company Name: _____

Address: _____

City, State, and Zip Code: _____

Authorized Signature: _____

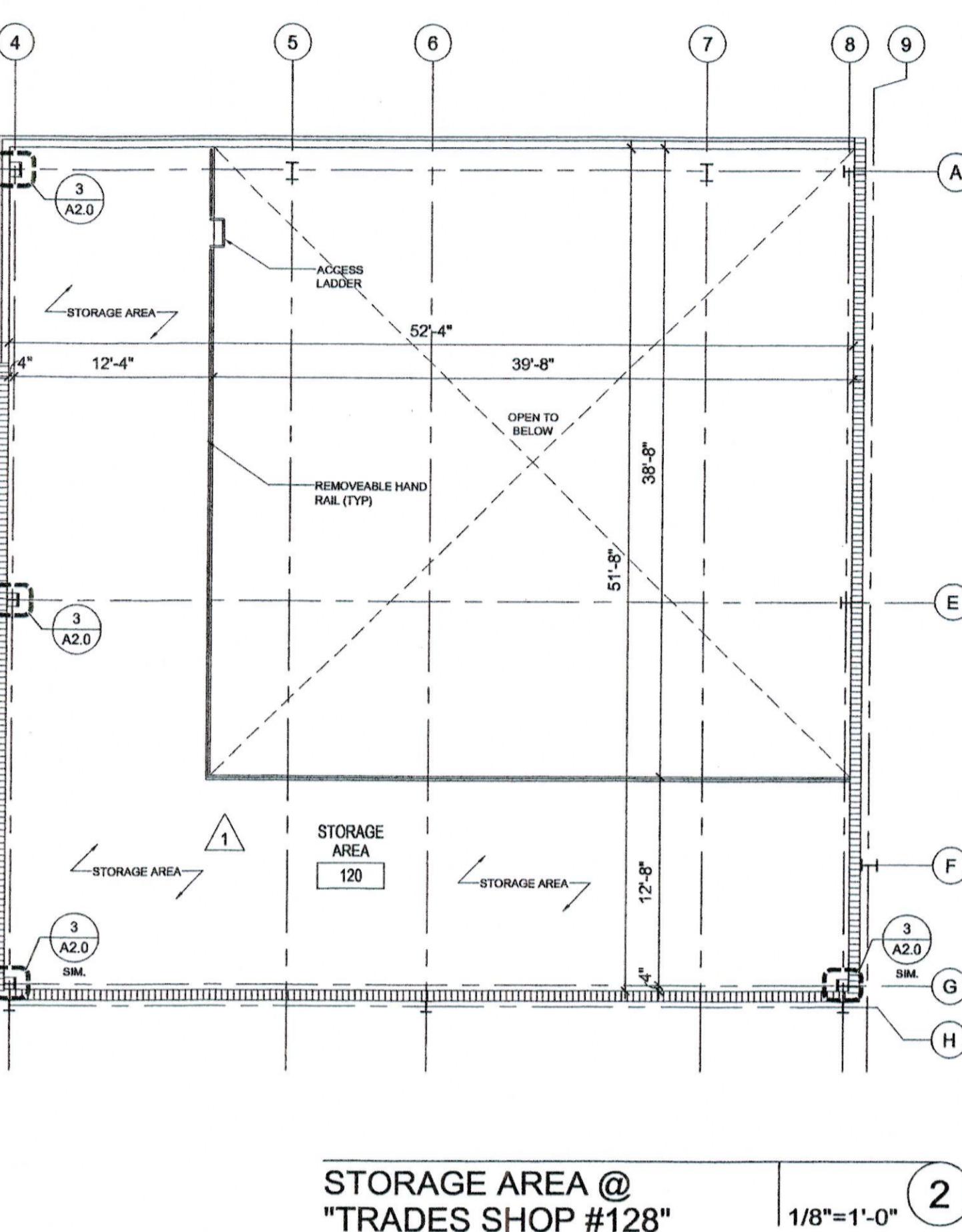
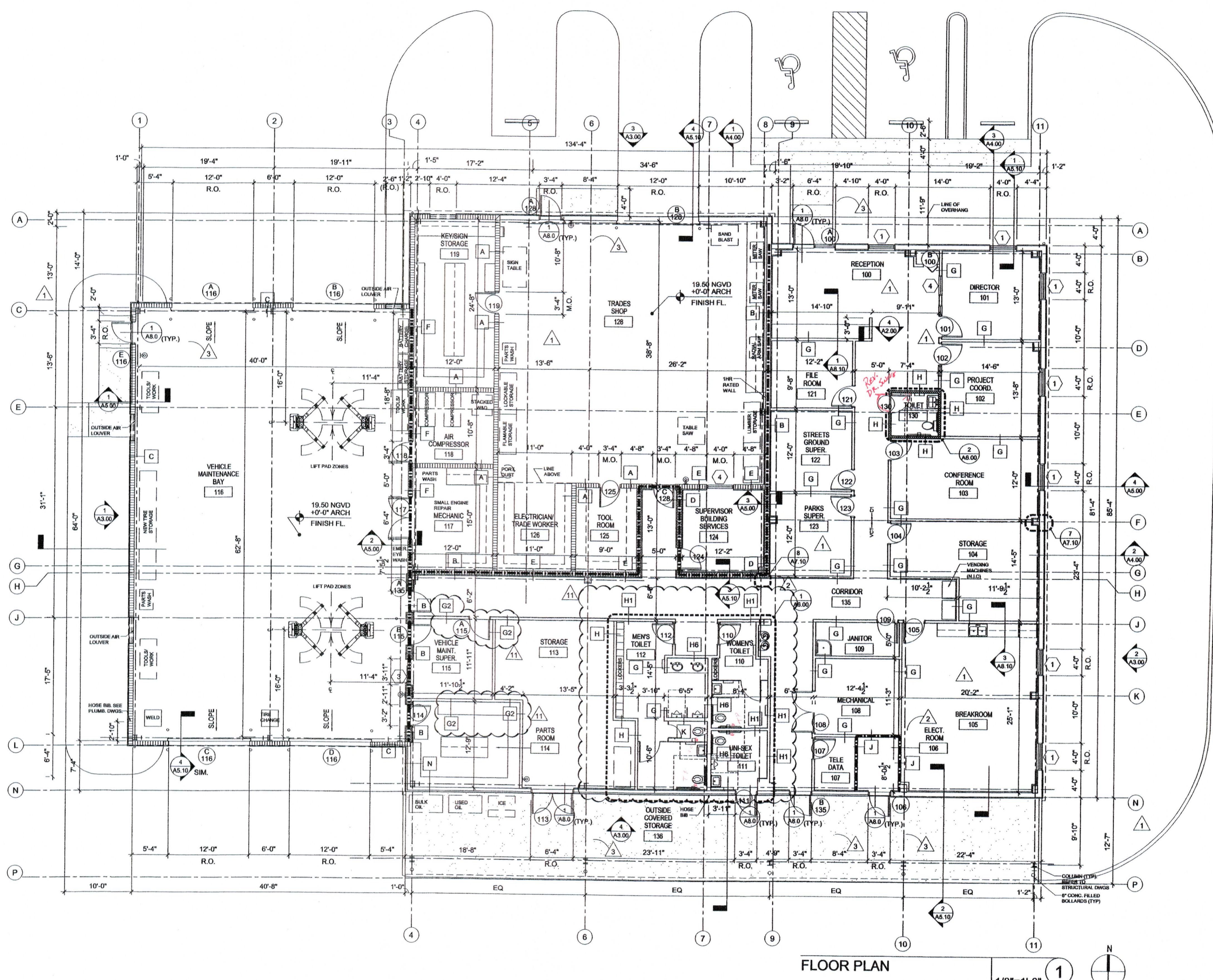
Name and Title: _____

Telephone: _____ Date: _____



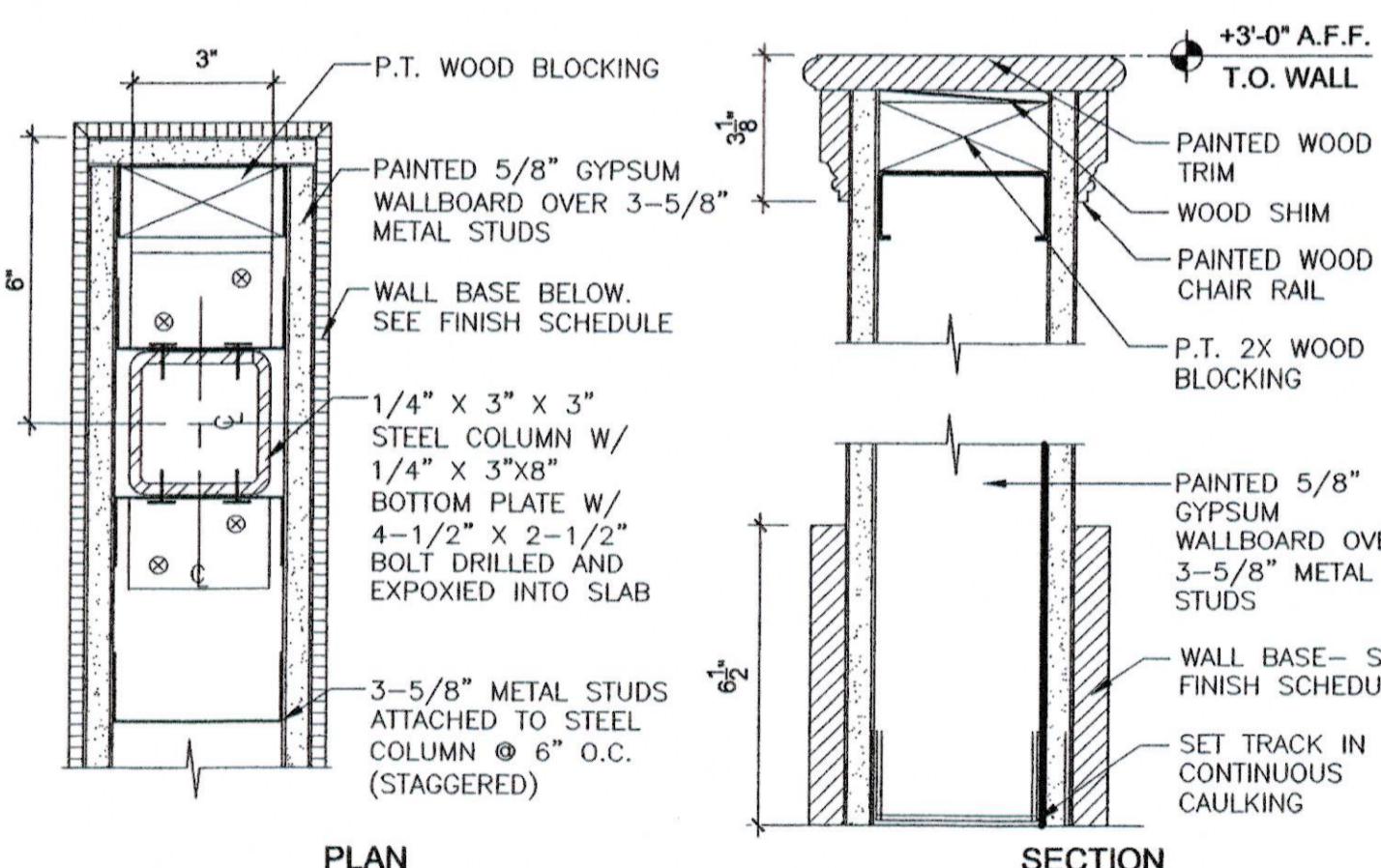
**CITY OF GREENACRES-PUBLIC WORKS
5750 MELALEUCA LANE
GREENACRES, FLORIDA
CONSTRUCTION DOCUMENTS**

FLOORPLAN



PRE-FAB. METAL BUILDING WALL
LEAVE 1" MIN
OPENING BETWEEN
SLAB AND COLUMN
COLUMN (TYP)
CONC. SLAB (TYP)

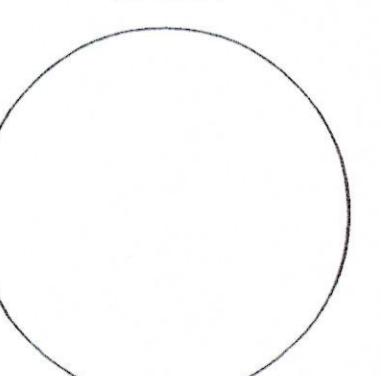
**OPENING IN SLAB @
STORAGE AREA COLUMNS** 3/4" = 1'-0" 3



LOW WALL DETAIL
PLAN & SECTION

BUILDING PERMIT

Project Number: 02041.00
Date Issued: 12/16/04



TAG LEGEND

SECTION 0000 ROOM NAME
0 0000 WINDOW TYPE
ELEVATION 0 0000 DOOR TYPE
0 0000 DETAIL X WALL TYPE

Joby L. Ballint, AR 16653

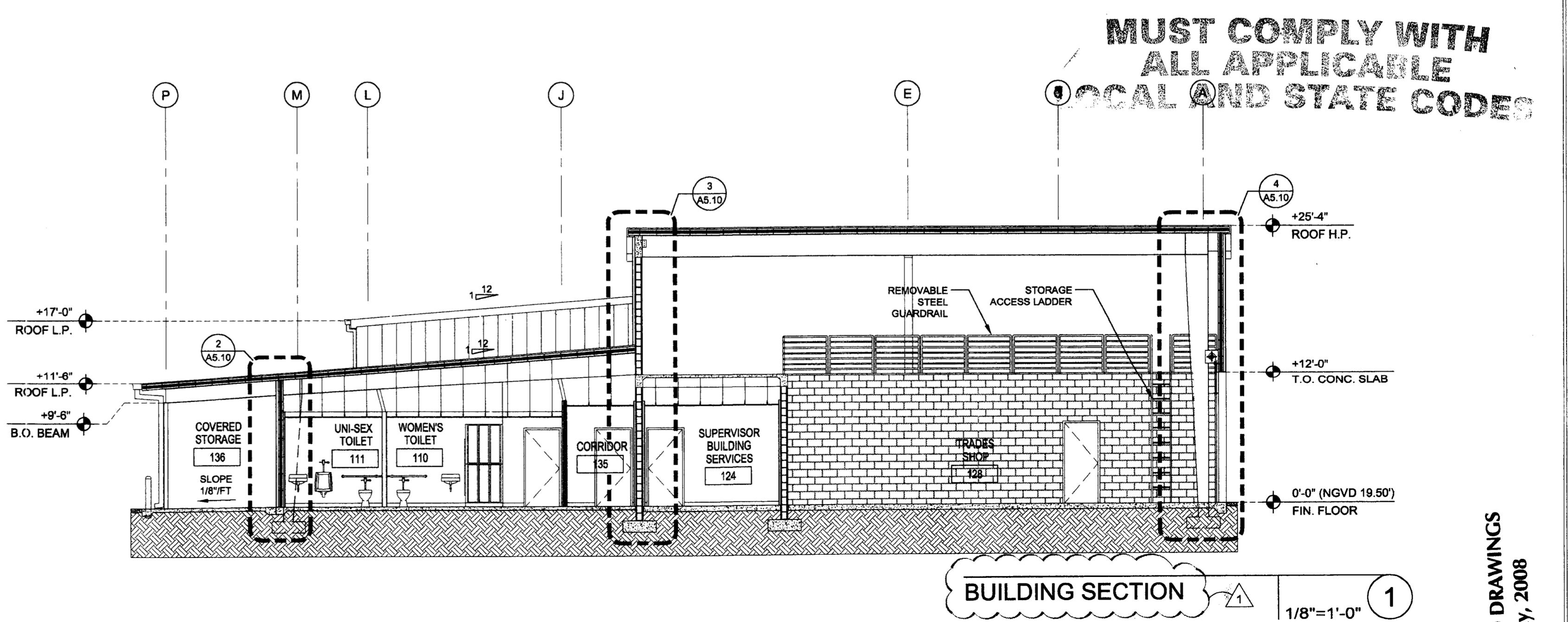
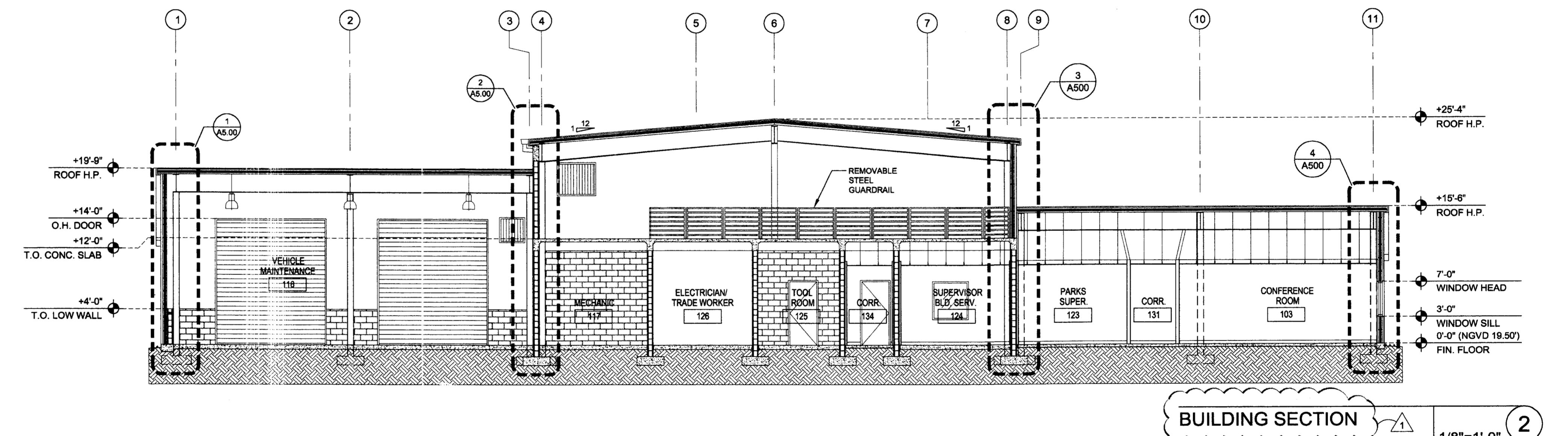
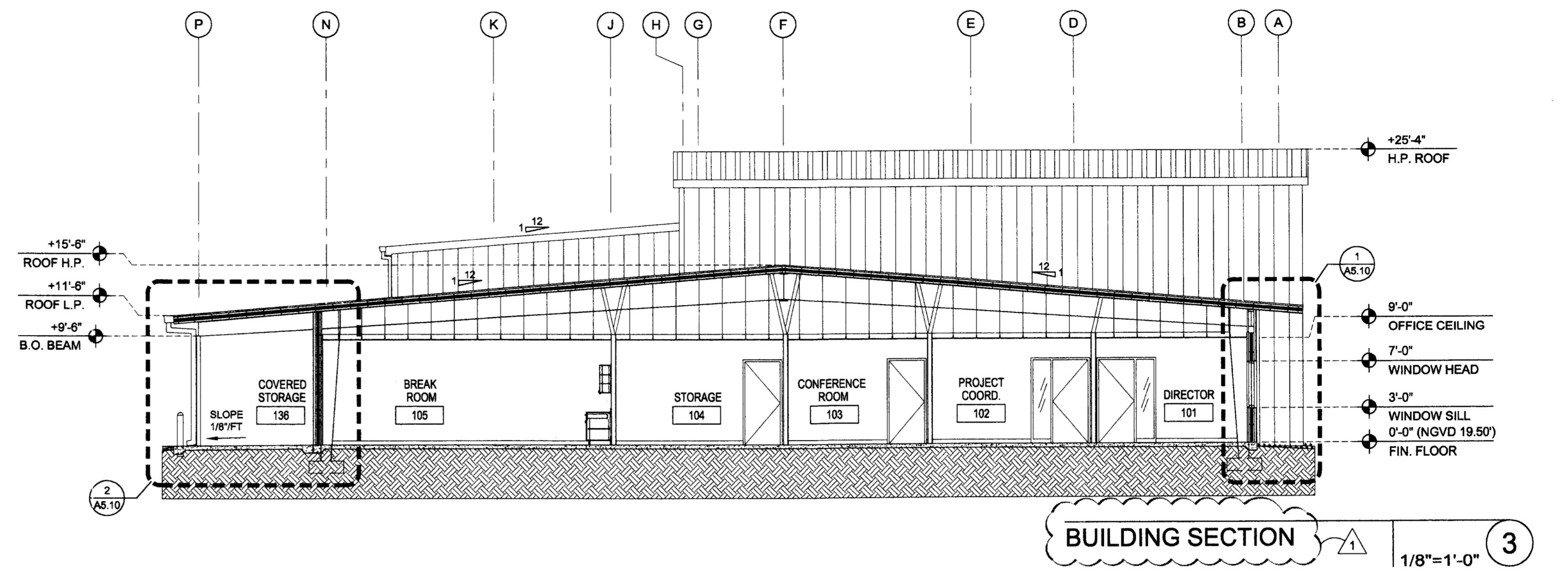
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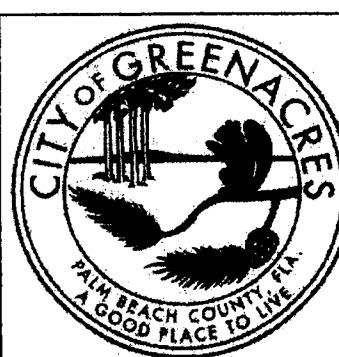
A2.00 PW
C36



CITY OF GREENACRES - PUBLIC WORKS
5750 MELALEUCA LANE
GREENACRES, FLORIDA
CONSTRUCTION DOCUMENTS

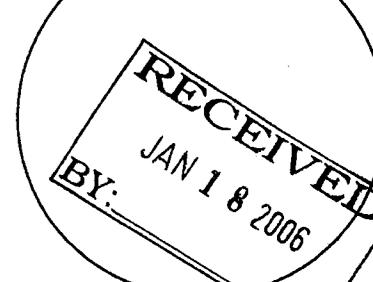
RECEIVE
AUG 04 2005
PBG




 CITY OF GREENACRES - CITY HALL
 5800 MELALEUCA LANE
 GREENACRES, FLORIDA
 CONSTRUCTION DOCUMENTS

 RECORD DRAWINGS
 May, 2008

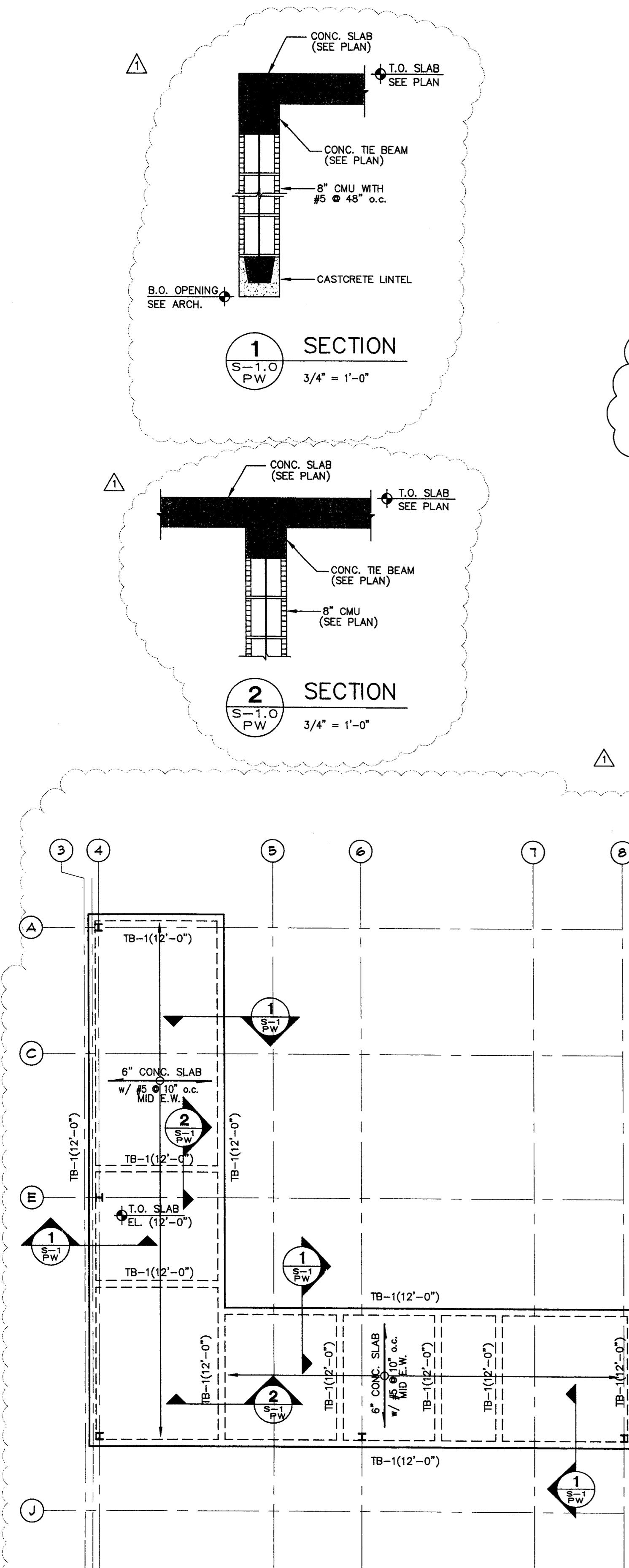
 RECEIVED by
 CITY OF GREENACRES
 PLANNING DEPARTMENT
 JAN 17 2006

 BUILDING PERMIT
 Project Number:
 02041.00
 Date Issued:
 12/15/04

 ONMJ
 O'Donnell, Naccarato, Mignogna & Jackson
 STRUCTURAL ENGINEERS

 321 15TH STREET, SUITE 200
 WEST PALM BEACH, FLORIDA 33401
 (561) 835-9994 FAX (561) 835-8255
 Job # 252.031

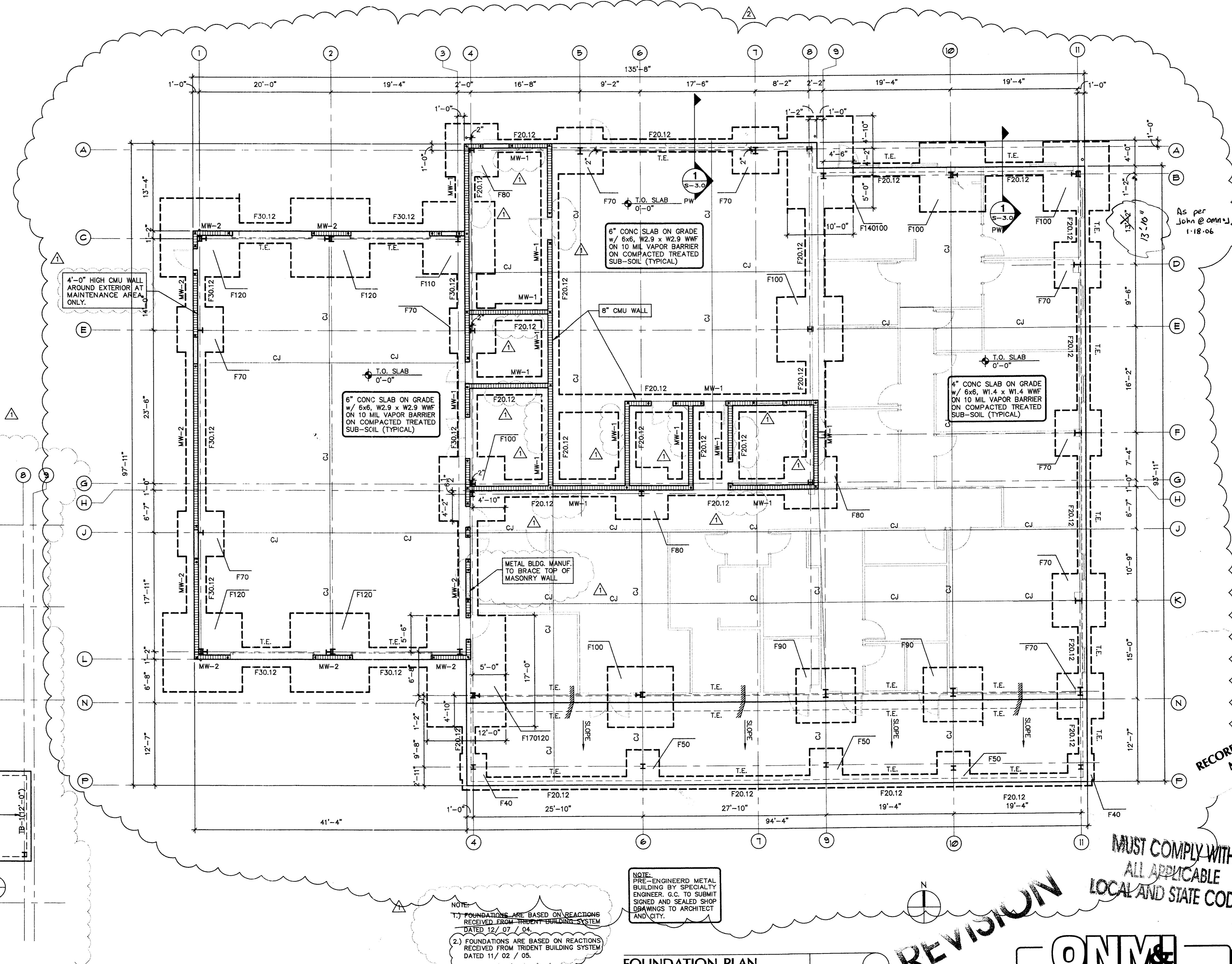
Sheet Number:

S-1.0 PW



MEZZANINE PLAN

1/8" = 1'-0"

 1.) DIRECTLY ABOVE OPENINGS PROVIDE A CASTCRETE LINTEL AS DIRECTED IN 10-A. IN THE STRUCTURAL NOTES ON S-2 PW.
 2.) (XX' - XX') INDICATES TOP OF BEAM ELEVATION.


FOUNDATION PLAN

1/8" = 1'-0"

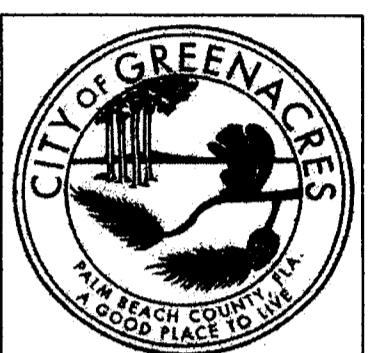
 NOTE:
 1.) FOUNDATIONS ARE BASED ON REACTIONS RECEIVED FROM TRIDENT BUILDING SYSTEM DATED 12/07/04.
 2.) FOUNDATIONS ARE BASED ON REACTIONS RECEIVED FROM TRIDENT BUILDING SYSTEM DATED 11/02/05.

 ONMJ
 O'Donnell, Naccarato, Mignogna & Jackson
 STRUCTURAL ENGINEERS

 321 15TH STREET, SUITE 200
 WEST PALM BEACH, FLORIDA 33401
 (561) 835-9994 FAX (561) 835-8255
 Job # 252.031

Sheet Number:

S-1.0 PW



**CITY OF GREENACRES - CITY HALL
5800 MELALEUCA LANE
GREENACRES, FLORIDA**

CONSTRUCTION DOCUMENTS

RECEIVED by
CITY OF GREENACRES
PLANNING DEPARTMENT

JAN 17 2006

BUILDING PERMIT

Project Number:
02041.00
Date Issued:
12/15/04



ian A. Nestler, AR 12428

Sheet Number:

S-2.0 PW

STRUCTURAL NOTES

CONTRACTOR NOTE:
THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL PREPARATIONS AND PROGRAMS IN CONNECTION WITH THE WORK. O'DONNELL, NACCARATO, MIGNOGNA & JACKSON, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS:

- WIND LOADS AS PER FLORIDA BUILDING CODE 2001 EDITION, FOR A 140 MPH WIND SPEED, EXPOSURE B, +/- 0.10 WIND PRESSURE COEFFICIENT, 1.0 IMPORTANCE FACTOR, AND BUILDING CATEGORY II. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.
- THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
 - FLORIDA BUILDING CODE 2001 EDITION.
 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318 / 2002 EDITION).
 - MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315 / LATEST EDITION).
 - MANUAL OF STANDARD PRACTICE FOR WELDING REINFORCING STEEL, INSERTS & CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION, AWS. D1.4 / 1992 EDITION.
 - SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) AISI ASD / 9TH EDITION OR LRFD 3RD EDITION.
 - SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 318 / LATEST EDITION.
 - BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, 5/TSM 402, 8/7/02 EDITIONS).

ARCHITECTURAL AND MECHANICAL DRAWINGS:

- THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND ARE NOT TO BE CONSIDERED AS A SUBSTITUTION FOR INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
- REFER TO ARCHITECTURAL, MECHANICAL, OR ELECTRICAL DRAWINGS FOR DIMENSIONS, FINISHES, INSERTS, BOLTS, SETTINGS, DRAINS, REGLES, ETC.
- BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED FOR ANY WORK THAT IS NOT PROPERLY SIZE BY THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
- DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
- SECTIONS AND DETAILS:
- ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.
- THRESHOLD INSPECTIONS SHALL BE PERFORMED DURING CONSTRUCTION OF THIS BUILDING AS REQUIRED BY SECTION 105.13 OF FBC.
- MATERIALS AND ASSEMBLY TEST AS FOLLOWS:
- EXTERIOR WINDOWS, SLIDING AND PATIO GLASS DOORS SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY, AND SHALL BE APPROVED WITH APPROVED LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS, TESTED BY APPROVED INDEPENDENT TEST AGENCY, TESTING LABORATORY, EVALUATION ENTITY OR MIAMI-Dade OR BROWARD COUNTY INSPECTOR TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF ONE OF THE FOLLOWING SPECIFICATIONS:
ANSI/AMSA/NWINDA 101/AS 2-97 OR TAS 202 (HWF SHALL COMPLY WITH TAS 202).
- EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED FOR STRUCTURAL INTEGRITY IN ACCORDANCE WITH ASTM E350 AT A LOAD OF 1.5 TIMES THE REQUIRED DESIGN PRESSURE LOAD. THE LOAD SHALL BE SUSTAINED FOR 10 SECONDS IN EXCESS OF 0.4 SPAN. AFTER THE LOAD IS REMOVED, HWF SHALL COMPLY WITH TAS 202, AFTER EACH SPECIFIED LOADING. CONTRACTOR SHALL NOT BE HELD RESPONSIBLE FOR DAMAGE TO FASTENERS, HARDWARE, OR ANY OTHER DAMAGE WHICH CAUSES THE DOOR TO BE INOPERABLE.
- SECTIONAL GARAGE DOORS SHALL BE TESTED FOR DETERMINATION OF STRUCTURAL PERFORMANCE UNDER FLORIDA STATIC AIR PRESSURE DIFFERENCE IN ACCORDANCE WITH ANSI/DASMA 108 OR TAS 202 (HWF SHALL COMPLY WITH TAS 202).
- CUSTOM (ONE OF A KIND) EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED BY AN APPROVED TESTING LABORATORY OR BE ENGINEERED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICES.
- WINDOW AND DOOR ASSEMBLIES SHALL BE ANCHORED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS TO ACHIEVE THE DESIGN PRESSURE SPECIFICATIONS. SUBSTITUTE ANCHORING SYSTEMS OR METHODS NOT SPECIFIED BY THE FENSTRATION MANUFACTURER SHALL PROVIDE EQUAL OR BETTER ANCHORING PERFORMANCE AS DEMONSTRATED BY ACCEPTED ENGINEERING PRACTICES.

SPECIAL ENGINEERED PRODUCTS

- THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED PRODUCTS. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE SUBMITTED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED AND APPROVED BY THE ENGINEER PRIOR TO RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR APPROXIMATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATENT LOADS. INTERIOR LOADS, EXTERIOR LOADS, AND LOADS FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT NOT LIMITED TO, THE FOLLOWING:

- LIGHT GAUGE METAL, INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.
- MISCELLANEOUS METALS INCLUDING: STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORT FRAMES THAT SUPPORT MACHINERY, PLATES FOR OTHER STRUCTURAL METAL USED FOR STRUCTURAL SYSTEMS.
- MISCELLANEOUS HANGERS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, SAFETY RAILINGS, GLAZING FRAMES, CLADDING SUCH AS PRECAST, ALUMINUM, METAL PANELS, CABLE BARREER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

FOUNDATION

- ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE REPORT ON SOILS AND FOUNDATION INVESTIGATION PREPARED BY NI, DATE 2.
- THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED ON THE DRAWINGS. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
- BOTTOM OF FOOTINGS TO BEAR ON CONTROLLED COMPACTED FILL CAPABLE OF SAFELY SUPPORTING 2500 PSF.
- BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2500 PSF.
- SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
- TOP OF ALL EXTERIOR FOOTINGS SHALL BE MINIMUM 1'-4" BELOW EXTERIOR FINISH GRADE.
- EXCAVATION & BACKFILL:
 - ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST COLLAPSE OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
 - PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
- CENTERLINE OF FOOTINGS: SHALL CONCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.
- DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECT AND ENGINEER. THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION, DISCUSSIONS SHALL BE Brought TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

- CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:

A. FOUNDATIONS	3000 PSI
B. SLAB-ON-GRADE	3000 PSI
C. TIE BEAMS	4000 PSI
D. MASONRY GROUT	4000 PSI

ALL OTHER CONCRETE TO BE 4000 PSI UNLESS NOTED OTHERWISE.

- ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

- A. MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS [AND HAVE A MINIMUM OF 317 LBS. OF CEMENT PER CUBIC YARD].
- B. A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI @ 28 DAYS [AND HAVE A MINIMUM OF 587 LBS. OF CEMENT PER CUBIC YARD].
- C. SLUMPS SHALL BE 3 MINIMUM AND 5 MAXIMUM.
- D. CONCRETE SHALL HAVE 3 PERCENT +/- 1.5 PERCENT AIR ENTRAINMENT.
- E. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
- F. CONCRETE WATER SHALL NOT BE ADDED.

- G. JOBSITE WATER SHALL NOT BE ADDED.

- H. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318 / 2002 EDITION), THE ACI DETAILING MANUAL (ACI 316 / 2002 EDITION) AND THE ACI DESIGN MANUAL FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 310 / LATEST EDITION).

- I. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

- J. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

- K. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 185, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.

- L. REQUIREMENTS:

- A. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.
- B. WWF SHALL COMPLY WITH ASTM A 185.

- C. LAP ALL BARS MINIMUM 48 DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 8 INCHES (UNLESS OTHERWISE NOTED).

- D. REINFORCING BARS:

- A. AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FABRIC SHALL BE HORIZONTAL, HORIZONTAL BARS 2'-5" BENT BAR FOR EACH CORNER, BARS BENT AT EACH FACE.
- B. ALL HOOKS SHOWN IN REINFORCING SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.

- C. FOR BALCONIES, SLADS AND WALKWAYS EXPOSED TO WEATHER, ALL REINFORCING STEEL (TOP AND BOTTOM) AS WELL AS SPACERS AND OTHER DEVICES FOR SPACING, SUPPORTING AND FASTENING REINFORCING SHAPES SHALL CONFORM TO ASTM A 767. BOLSTERS AND CHAIRS TO BE PLASTIC CONCRETE PLACED IN THE SPACES HAVE 40 W/C RATIO MAXIMUM AND CONTAIN 2.5 GALLONS OF CALCIUM NITRATE PER CUBIC YARD. REBAR COVER TO BE 1.5" MINIMUM.

- D. CONTRACTOR SHALL INCLUDE IN HIS BASE BID THE COST OF 5,000 LBS. OF ALL REINFORCING STEEL, INCLUDING FABRICATION, FABRICATION, BENDING, FURNISHING, SHAPING, PLACING, THIS EXTRA STOCK IS FURNISHED AND USED FOR SPECIAL CONDITIONS AS DIRECTED BY THE ARCHITECT, THE ARCHITECT'S AGENT OR BY THE OWNER'S CONSTRUCTION SUPERVISOR. THE COST OF THE UNUSED EXTRA STOCK SHALL BE CREDITED TO THE OWNER'S ACCOUNT.

- E. CONCRETE LINTELS:

- A. DROP BOTTOM OF BEAM AT WINDOWS, DOORS AND MASONRY OPENINGS AS REQUIRED TO PROVIDE A CONCRETE CLOSURE BETWEEN THE BOTTOM OF THE BEAM AND WINDOW AND/OR DOOR HEADER OR PROVIDE A PLAIN CONCRETE LINTEL BY CASTCMENT IF NOT NEXT TO A POURED CONCRETE COLUMN.

- B. MAXIMUM DROP SHALL BE 16" (TWO BLOCK COURSES) AND SPAN EQUAL TO MASONRY OPENING WIDTH. PROVIDE 2 #5 AT BOTTOM OF DROP INCLUDING 3 TIES @ 24" O.C. EXTENDING TO TOP OF BEAM REINFORCING. REINFORCING SHALL BE 1/2" DIA. #5 BARS. IF SPAN OF DROP, A SEPARATED LINTEL SHALL BE PROVIDED AS FOLLOWS:

- L1. OPENING LESS THAN 6'-0" INDE 8'-0" W/ 8'-0" W/2 #5 BOTTOM BARS, L2. OPENING BETWEEN 6'-0" AND 12'-0" INDE 8'-0" W/2 #5 BOTTOM BARS.

- C. LINTELS TO HAVE 8" MINIMUM BEARING AT EACH END.

- D. IF THE MASONRY DESIGN HAS AN END ADJACENT TO A CONCRETE COLUMN, PROVIDE 2 #5 OR 3 #4 BARS. THE SPAN MAY BE IN THE CONCRETE COLUMN WITH SHEAR KEY 1-1/2" DEEP BY LINTELS DEPTH AND WIDTH FOR ITS SUBSEQUENT CONSTRUCTION.

- E. MASONRY UNITS SHALL BE LOAD BEARING ASTM C90, TYPE I, NORMAL WEIGHT WITH MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI ON NET AREA OF INDIVIDUAL UNITS. CMU SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND (U.A.C.).

- F. FOLLOWING ARE THE BLOCK STRENGTHS REQUIRED:

- A. ASTM C 90 - 2,500 PSI ON NET AREA OF INDIVIDUAL UNITS

- B. ALL MORTAR SHALL BE TYPE S OR M IN ACCORDANCE WITH ASTM SPECIFICATION C270 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 PSI AT 28 DAYS, (2500 WITH TYPE M), FROM FIELD OBTAINED TEST CUBES (MIN. OF TWO)

- C. GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476 HAVING A COMPRESSIVE STRENGTH OF 3,000 PSI FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)

- D. ALL CONCRETE MASONRY BEARING AND SHEAR WALLS SHALL BE INSPECTED BY A QUALIFIED ENGINEER AND CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY CONSTRUCTION" (ACI 530/ASCE 5/TSM 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 531/ASCE 6/TSM 602 / 2002 EDITIONS).

- E. PROVIDE 8" X 8" MASONRY BEAM WITH 2 #5 CONT. AT EVERY WINDOW SILL, EXTEND BEAM 8" BEYOND EDGE OF OPENING.

- F. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 18" ON CENTER VERTICAL IN ALL MASONRY WALLS, PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS.

- G. PROVIDE CONTROL JOINTS IN MASONRY WALLS AT A SPACING OF 30' + O.C. AND ALIGN WITH ARCHITECTURAL CONTROL JOINTS.

- H. EPOXY GROUT SHALL BE NON-SHRINK HIGH CREEP RESISTANT, AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES:

- I. CUBE STRENGTH AT 30' 1,500 PSI

- J. FLEXURE STRENGTH, ASTM C 580, 1,000 PSI

- K. COMPRESSIVE STRENGTH, ASTM C 579, 1,600 PSI/7 DAYS.

- L. TOP OF ALL EXTERIOR FOOTINGS SHALL BE MINIMUM 1'-4" BELOW EXTERIOR FINISH GRADE.

- M. EXCAVATION & BACKFILL:
 - A. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST COLLAPSE OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
 - B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.

- N. CENTERLINE OF FOOTINGS: SHALL CONCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.

- O. DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCUSSIONS SHALL BE Brought TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

MASONRY WALL SCHEDULE		
MARK	THICKNESS	REINFORCING
MW-1	8" CMU	#6 @ 40" o

