



**BID NO. 26-004
PUBLIC WORKS STEEL COLUMNS REPAIR**

ADDENDUM NO. 1

January 16, 2026

This addendum shall modify and become a part of the bid document. Specifications not specifically mentioned in this Addendum are not altered by any changes, amendments, deletions, or additions.

Reply to Requests for Information:

- 1. Could you provide the height of the beams to be repaired for project 26-004.**

The height of the columns and any other existing conditions are for the contractor to verify in the field during one of the mandatory pre-bid site visits

- 2. Could we please get a set of the as-built structural drawings.**

A complete set of as-built drawings is not available; bidders are to verify as-built conditions in the field during one of the mandatory pre-bid site visits. Attached are some record structural and architectural drawings of the building for information only; do not solely rely on these sheets for any take offs or other purpose; as it may not be an accurate representation of existing conditions.

- 3. Will AMPP QP1/QP2 be required for blasting and painting of the steel?**

No.

The proposal submission deadline is Friday, January 30, 2026, at 3:00 PM. Any proposal response received after the designated closing time will be returned unopened.

Any questions concerning this Addendum should be addressed to Monica Powery, Director of Purchasing at (561) 642-2039.

All Proposers shall acknowledge receipt of this addendum by annotating the proposal signature page with the addendum number, completing the section below, and returning with the proposal response to verify receipt.

Company Name: _____

Address: _____

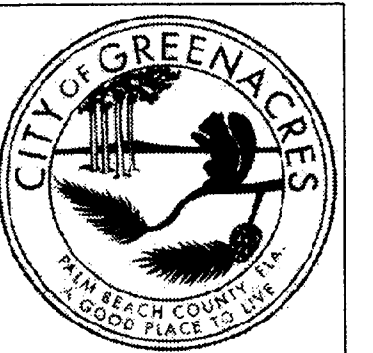
City, State, and Zip Code: _____

Authorized Signature: _____

Name and Title: _____

Telephone: _____ Date: _____

A2.00 PW
CB6



CITY OF GREENACRES - PUBLIC WORKS
5750 MELALEUCA LANE
GREENACRES, FLORIDA
CONSTRUCTION DOCUMENTS

BUILDING SECTIONS

BUILDING PERMIT

Project Number:
02041.00
Date Issued:
12/15/04

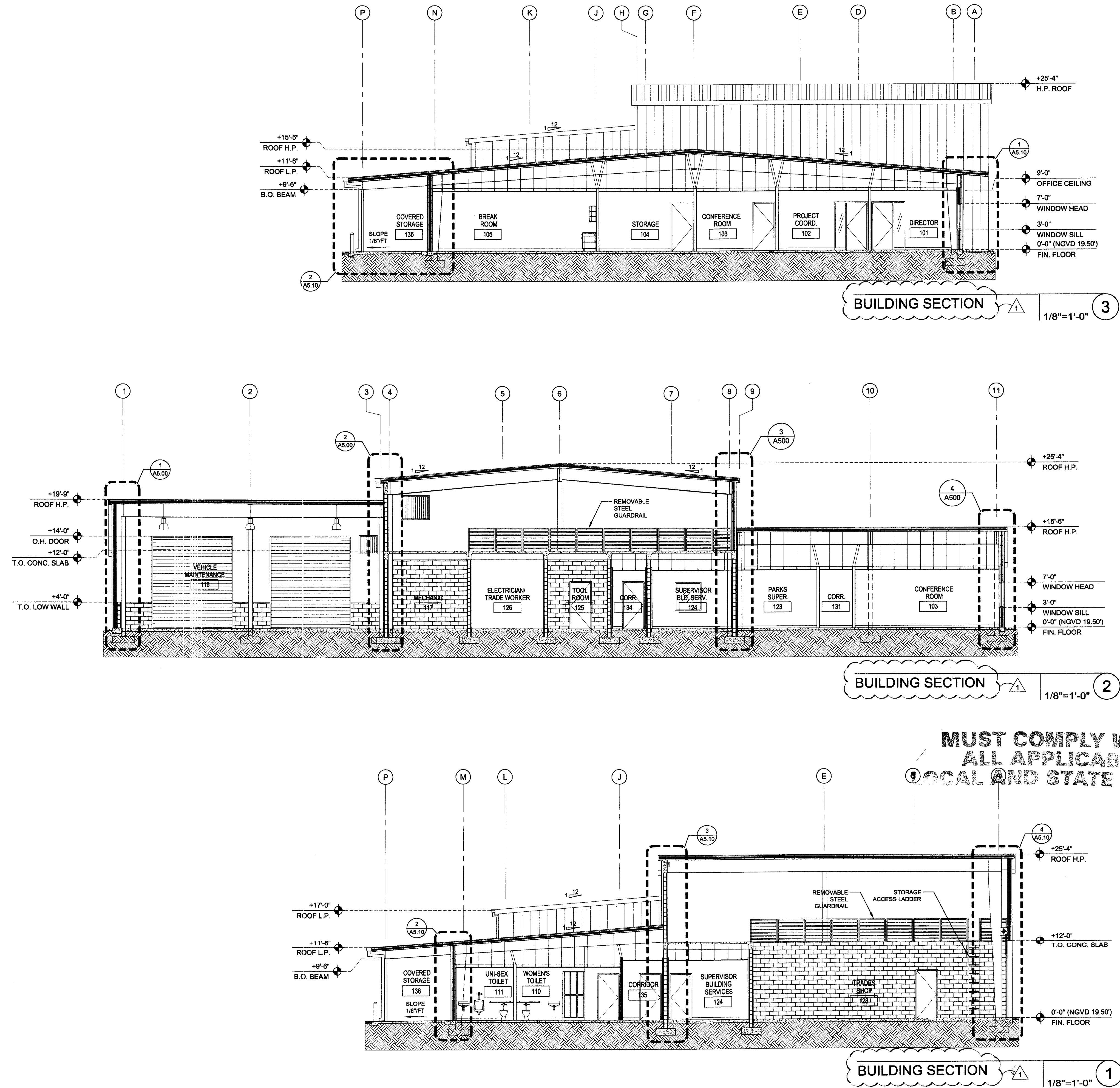
LAL
cluster

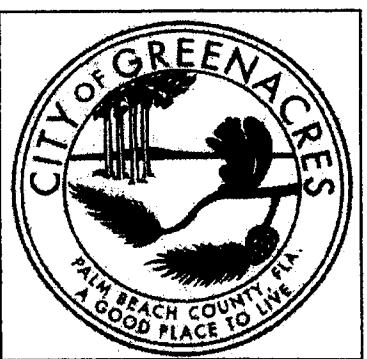
lan A. Nestler, AR 12428

Sheet Number:

A4.00 PW

C.B.6





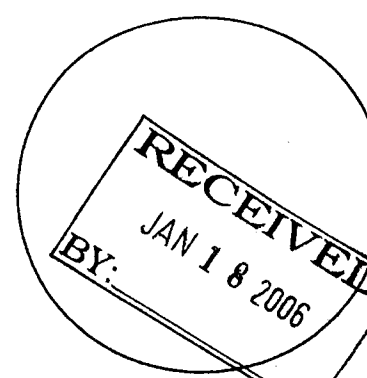
CITY OF GREENACRES - CITY HALL 5800 MELALEUCA LANE GREENACRES, FLORIDA CONSTRUCTION DOCUMENTS

RECORD DRAWINGS
May, 2008

RECEIVED by
CITY OF GREENACRES
PLANNING DEPARTMENT
JAN 17 2006

BUILDING PERMIT

Project Number:
02041.00
Date Issued:
12/15/04



Ian A. Nestler, AR 12428

Sheet Number:

S-1.0 PW

MUST COMPLY WITH
ALL APPLICABLE
LOCAL AND STATE CODES

REVISION

ONM

O'Donnell, Naccarato, Mignogna

STRUCTURAL ENGINEERS

221 15TH STREET, SUITE 200
WEST PALM BEACH, FLORIDA 33411
(561) 835-9994 FAX (561) 835-8255

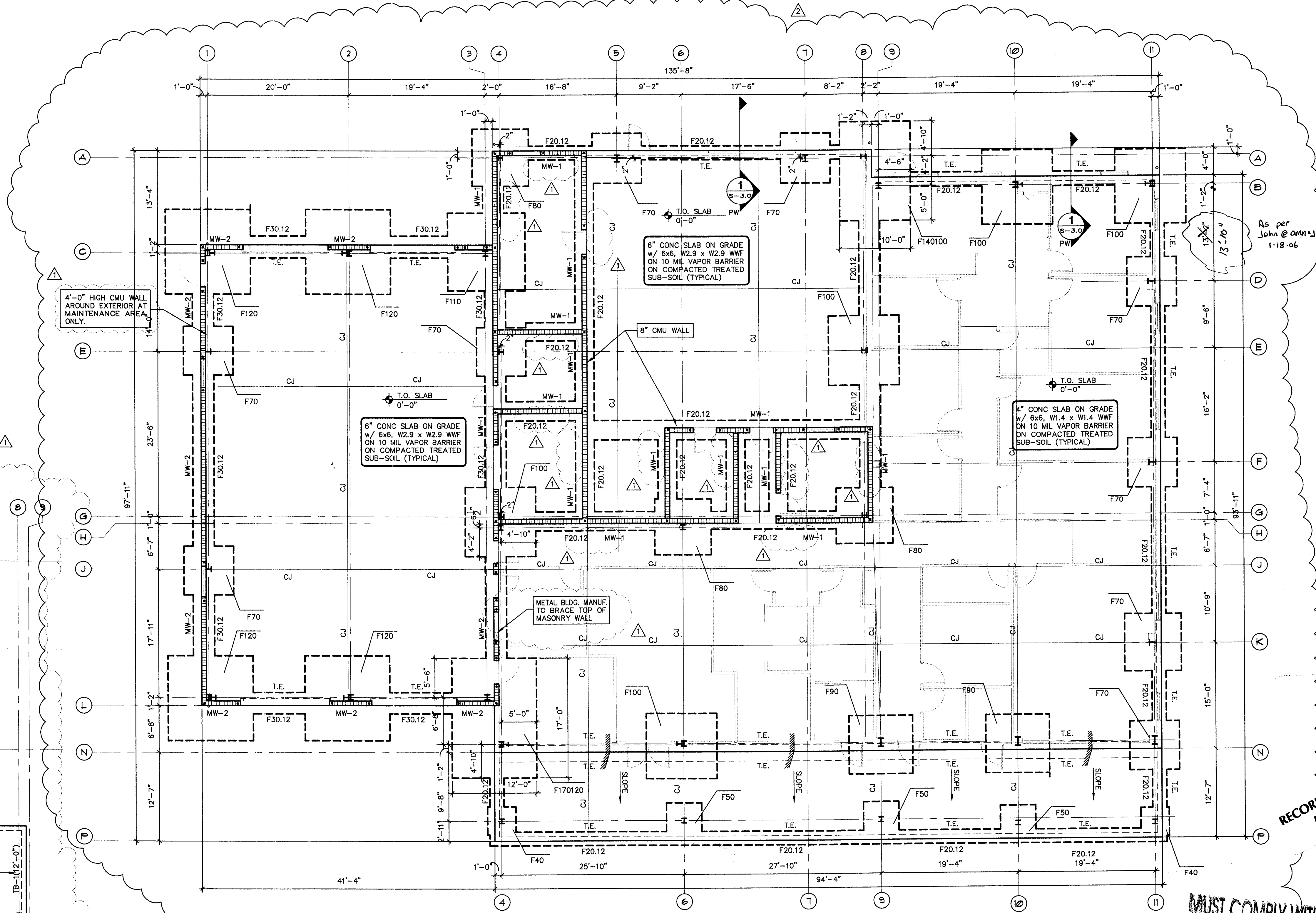
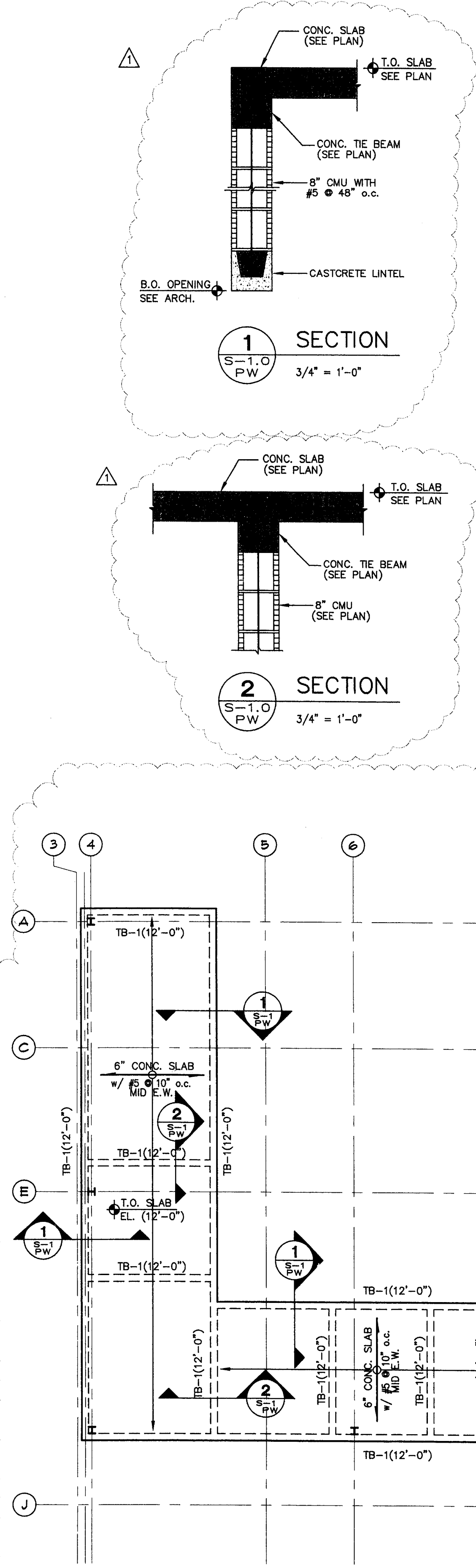
LOC. #0004386
JOB # 252.031

FOUNDATION PLAN

1/8" = 1'-0"

NOTE:
1. FOUNDATIONS ARE BASED ON REACTIONS
RECEIVED FROM TRIDENT BUILDING SYSTEM
DATED 12/07/04.
2. FOUNDATIONS ARE BASED ON REACTIONS
RECEIVED FROM TRIDENT BUILDING SYSTEM
DATED 11/02/05.

NOTE:
PRE-ENGINEERED METAL
BUILDING BY SPECIALTY
ENGINEER, G.C. TO SUBMIT
SIGNED AND SEALED SHOP
DRAWINGS TO ARCHITECT
AND CITY.





CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. O'DONNELL, NACCARATO, MIGNOGNA & JACKSON, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS

- WIND LOADS AS PER FLORIDA BUILDING CODE 2001 EDITION, FOR A 140 MPH WIND SPEED, EXPOSURE B, +/- 0.18 INTERNAL PRESSURE COEFFICIENT, 1.0 IMPORTANCE FACTOR, AND BUILDING CATEGORY II. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.
- THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
 - FLORIDA BUILDING CODE 2001 EDITION.
 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ 2002 EDITION).
 - MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).
 - MANUAL OF STANDARD PRACTICE FOR WELDING REINFORCING STEEL, INSERTS & CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION, AWS. D1.4/ 1992 EDITION.
 - SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) AISC ASD/ 9TH EDITION OR LRPD 2ND EDITION.
 - SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 301/LATEST EDITION.
 - BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, 6/TMS 402, 602/2002 EDITIONS).

ARCHITECTURAL AND MECHANICAL DRAWINGS:

- THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
- REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, INSERTS, BOLTS SETTINGS, DRAINAGE, REGISTERS, ETC.
- BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.

- DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.

SECTIONS AND DETAILS:

ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

- THRESHOLD INSPECTIONS SHALL BE PERFORMED DURING CONSTRUCTION OF THIS BUILDING AS REQUIRED BY SECTION 105.13 OF FBC.

MATERIALS AND ASSEMBLY TEST AS FOLLOWS:

- EXTERIOR WINDOWS, SLIDING AND PATIO GLASS DOORS SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY, AND SHALL BE PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT CERTIFICATION AGENCY, TESTING LABORATORY, EVALUATION ENTITY OR MIAMI-DADE NOTICE OF ACCEPTANCE TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF ONE OF THE FOLLOWING SPECIFICATIONS:
 - ANSI/AAMA/NWMA 101/1.3. 2-97 OR TAS 202 (HVHZ SHALL COMPLY WITH TAS 202).
- EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED FOR STRUCTURAL INTEGRITY IN ACCORDANCE WITH ASTM E330 AT A LOAD OF 1.5 TIMES THE REQUIRED DESIGN PRESSURE LOAD. THE LOAD SHALL BE SUSTAINED FOR 10 SECONDS WITH NO PERMANENT DEFORMATION OF ANY MAIN FRAME OR PANEL MEMBER. HVHZ SHALL COMPLY WITH TAS 202. AFTER EACH SPREAD IS REMOVED, THERE SHALL BE NO GLASS BREAKAGE, PERMANENT DAMAGE TO FASTENERS, HARDWARE PARTS, OR ANY OTHER DAMAGE, WHICH CAUSES THE DOOR TO BE INOPERABLE.
- SECTIONAL GARAGE DOORS SHALL BE TESTED FOR DETERMINATION OF STRUCTURAL PERFORMANCE UNDER UNIFORM STATIC AIR PRESSURE DIFFERENCE IN ACCORDANCE WITH ANSI/DASMA 108 OR TAS 202 (HVHZ SHALL COMPLY WITH TAS 202).
- CUSTOM (ONE OF A KIND) EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED BY AN APPROVED TESTING LABORATORY OR BE ENGINEERED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICES.
- WINDOW AND DOOR ASSEMBLIES SHALL BE ANCHORED IN ACCORDANCE WITH THE PUBLISHED MANUFACTURER'S RECOMMENDATIONS TO ACHIEVE THE DESIGN PRESSURE SPECIFIED. SUBSTITUTE ANCHORING SYSTEM USED FOR SUBSTRATES NOT SPECIFIED BY THE PENSTRATION MANUFACTURER SHALL PROVIDE EQUAL OR GREATER ANCHORING PERFORMANCE AS DEMONSTRATED BY ACCEPTED ENGINEERING PRACTICE.

SPECIALTY ENGINEERED PRODUCTS

- THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
 - LIGHT GAUGE METAL INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILING, ETC.
 - MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.
 - MISCELLANEOUS HANGERS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, SAFETY RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

FOUNDATION

- ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE REPORT ON SOILS AND FOUNDATION INVESTIGATION PREPARED BY NI, DATED 02.
- THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
- BOTTOM OF FOOTINGS TO BEAR ON CONTROLLED COMPACTED FILL CAPABLE OF SAFELY SUPPORTING 2500 PSF.
- BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2500 PSF.
- SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING CONSTRUCTION. IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
- TOP OF ALL EXTERIOR FOOTINGS SHALL BE MINIMUM 1'-4" BELOW EXTERIOR FINISH GRADE.
- EXCAVATION & BACKFILL:
 - ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
 - PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
- CENTERLINE OF FOOTINGS: SHALL COINCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.
- DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:

- FOUNDATIONS 3000 PSI
- SLAB-ON-GRADE 3000 PSI
- BEAMS 4000 PSI
- MASONRY GROUT 4000 PSI

ALL OTHER CONCRETE TO BE 4000 PSI UNLESS NOTED OTHERWISE.

ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

- A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS [AND HAVE A MINIMUM OF 917 LBS. OF CEMENT PER CUBIC YARD].
- A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI @ 28 DAYS [AND HAVE A MINIMUM OF 587 LBS. OF CEMENT PER CUBIC YARD].
- SLUMPS SHALL BE 3 MINIMUM AND 5 MAXIMUM.
- CONCRETE SHALL HAVE 3 PERCENT +/- 1.5 PERCENT AIR ENTRAINMENT.
- ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
- JOBSITE WATER SHALL NOT BE ADDED.

- ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ 2002 EDITION), THE ACI DETAILING MANUAL (ACI 315/ 1994 EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).

- SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

- WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 185, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.

REQUIREMENTS:

- ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM SPECIFICATION A 615 GRADE 60.
- WVF SHALL COMPLY WITH ASTM A 185.
- LAP ALL BARS MINIMUM 48 DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WVF A MINIMUM OF 6 INCHES (UNLESS OTHERWISE NOTED).
- REINFORCING BARS:
 - AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE MATCHING HORIZONTAL BARS X 5'-0" BENT BAR FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE.
 - ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.
 - FOR BALCONIES, SLABS AND WALKWAYS EXPOSED TO WEATHER ALL REINFORCING STEEL (TOP AND BOTTOM) AS WELL AS SPACERS AND OTHER DEVICES FOR SPACING, SUPPORTING AND FASTENING REINFORCING SHALL BE GALVANIZED CONFORMING TO ASTM A 797. BOLSTERS AND CHAIRS TO BE PLASTIC. CONCRETE PLACED IN THESE AREAS TO HAVE .40 W/C RATIO MAXIMUM AND CONTAIN 2.5 GALLONS OF CALCIUM NITRATE PER CUBIC YARD. REBAR COVER TO BE 1.5" MINIMUM.
- CONTRACTOR SHALL INCLUDE IN HIS BASE BID THE COST OF 5,000 LBS. OF ADDITIONAL REINFORCING STEEL, INCLUDING DETAILING, FABRICATION, BENDING, FURNISHING, AND PLACING. THIS EXTRA STOCK SHALL BE FURNISHED AND USED FOR SPECIAL CONDITIONS AS DIRECTED BY THE ARCHITECT. THE ARCHITECT'S AGENT OR BY THE OWNER'S CONSTRUCTION SUPERVISOR. THE PRICE OF THE UNUSED EXTRA STOCK SHALL BE CREDITED TO THE OWNER'S ACCOUNT.

MASONRY

- DROP BOTTOM OF BEAM AT WINDOWS, DOORS AND MASONRY OPENINGS AS REQUIRED TO PROVIDE A CONCRETE CLOSURE BETWEEN THE BOTTOM OF THE BEAM AND WINDOW AND/OR DOOR HEADER OR PROVIDE A PRECAST CONCRETE LINTEL BY CASTORETE IF NOT NEXT TO A POURED CONCRETE COLUMN.
- MAXIMUM DROP SHALL BE 16" (TWO BLOCK COURSES) AND SPAN EQUAL TO MASONRY OPENING WIDTH. PROVIDE 2 #5 AT BOTTOM OF DROP INCLUDING 45 TIES @ 24" O.C. EXTENDING TO TOP OF BEAM REINFORCING. IF THE LINTEL EXCEEDS THE ABOVE LIMIT OF DROP, A SEPARATED LINTEL SHALL BE PROVIDED AS FOLLOWS:
 - OPENING LESS THAN 6'-0" WIDE 8" X 8" W/2 #5 BOTTOM BARS.
 - OPENING BETWEEN 6'-0" AND 12'-0" 8" X 12" W/2 #5 BOTTOM BARS.
- LINELS TO HAVE 8" MINIMUM BEARING AT EACH END.
- IF THE MASONRY OPENING HAS AN END ADJACENT TO A CONCRETE COLUMN PROVIDE (2) #5 OR #6 DOWELS, AS THE CASE MAY BE, IN THE CONCRETE COLUMN WITH SHEAR KEY 1-1/2 INCH DEEP BY LINTEL'S DEPTH AND WIDTH FOR ITS SUBSEQUENT CONSTRUCTION.

MASONRY WALL SCHEDULE		
MARK	THICKNESS	REINFORCING
MW-1	8" CMU	#5 @ 40" o.c.
MW-2	8" CMU	#5 @ 32" o.c.

MASONRY WALL NOTES:

- WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 8" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
- ADJACENT TO ANY EXTERIOR WALL OPENING, PLACE 1 # 5 VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT.
- ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.

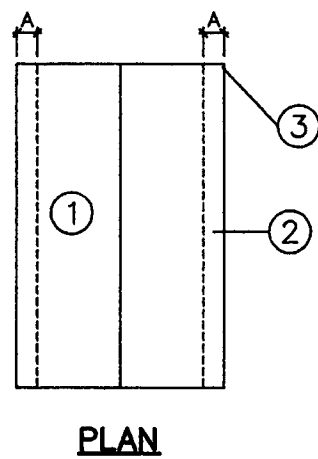
CONCRETE BEAM SCHEDULE						
MARK	SIZE BxH (Inches)	REINFORCEMENT		STIRRUPS		REMARKS
		BOTT	TOP	INT	SPACING	
TB-1	8 x 12	2 #5	2 #5	-	#3 @24" o.c., @ 4" o.c. OVER OPENINGS	-

ROOF WIND PRESSURE (PSF) COMPONENTS AND CLADDING-EXPOSURE B-140 MPH WIND SPEED				
EFFECTIVE WIND AREA (SQ. FT.)	ROOF AREA			
	1	2	3	
10	+14.3 / -35.3	+14.3 / -59.2	+14.3 / -89.0	
20	+13.4 / -34.4	+13.4 / -52.9	+13.4 / -73.8	
50	+12.3 / -33.2	+12.3 / -44.5	+12.3 / -53.5	
100	+11.4 / -32.3	+11.4 / -38.2	+11.4 / -38.2	

IMPORTANCE FACTOR 1.0

NOTES:

- WIND DESIGN PER FBC-2001
- +1 INDICATES WIND PRESSURE
-1 INDICATES WIND SUCTION
- WALL DISTANCE A = 10 FT (COMPONENTS AND CLADDING)
- FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE WIND AREA.



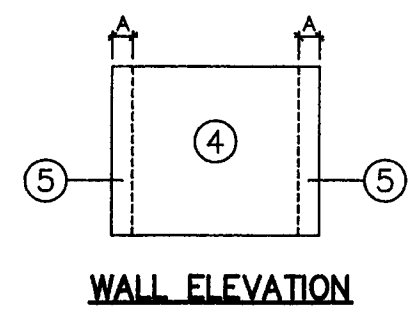
PLAN

DOOR & WINDOW WIND PRESSURE (PSF) COMPONENTS AND CLADDING-EXPOSURE B-140 MPH WIND SPEED		
SIZE OF WALL OPENING (SQ. FT.)	WALL AREA	
	4	5
10	+35.3 / -38.2	+35.3 / -47.2
20	+33.7 / -36.7	+33.7 / -44.0
50	+31.6 / -34.6	+31.6 / -39.8
100	+30.0 / -33.0	+30.0 / -36.7

IMPORTANCE FACTOR 1.0

NOTES:

- WIND DESIGN PER FBC-2001
- +1 INDICATES WIND PRESSURE
-1 INDICATES WIND SUCTION
- WALL DISTANCE A = 10 FT
- FOR WALL OPENING BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER WALL OPENING AREA.



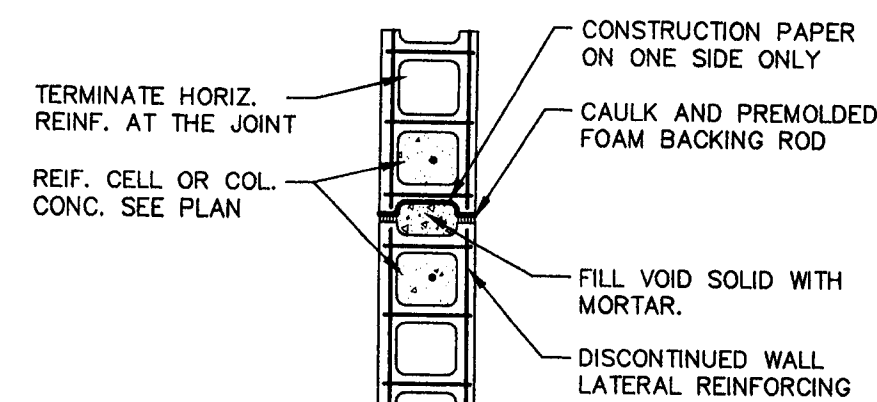
WALL ELEVATION

FOOTING SCHEDULE PUBLIC WORKS		
MARK	SIZE	REINFORCING
F2012	2'-0" x CONT. x 1'-0"	3 #5 CONT. #5 @ 24" o.c. TRANS. BOTT.
F3012	3'-0" x CONT. x 1'-0"	4 #5 CONT. & #5 @ 16" o.c. TOP & BOTT.
F70	7'-0" x 7'-0" x 1'-6"	8 #6 E.W. TOP & BOTT.
F80	8'-0" x 8'-0" x 1'-6"	9 #6 E.W. TOP & BOTT.
F90	9'-0" x 9'-0" x 2'-0"	10 #6 E.W. TOP & BOTT.
F100	10'-0" x 10'-0" x 2'-0"	#6 @ 10" o.c. E.W. TOP & BOTT.
F115	11'-6" x 11'-6" x 2'-6"	#7 @ 10" o.c. E.W. TOP & BOTT.
F120	12'-0" x 12'-0" x 3'-0"	#8 @ 10" o.c. E.W. TOP & BOTT.
F110	11'-0" x 11'-0" x 2'-6"	#7 @ 10" o.c. E.W. TOP & BOTT.
F40	4'-0" x 4'-0" x 1'-6"	5 #6 E.W. TOP & BOTT.
F50	5'-0" x 5'-0" x 1'-6"	6 #6 E.W. TOP & BOTT.
F140100	14'-0" x 10'-0" x 3'-0"	#8 @ 10" o.c. E.W. TOP & BOTT.
F170120	17'-0" x 12'-0" x 3'-0"	#8 @ 10" o.c. E.W. TOP & BOTT.

REVISION

MUST COMPLY WITH
ALL APPLICABLE
LOCAL AND STATE CODES

ONM
O'Donnell, Naccarato, Mignogna & Jackson
STRUCTURAL ENGINEERS
321 15TH STREET, SUITE 200
WEST PALM BEACH, FLORIDA 33401
(561) 835-1994 FAX (561) 835-8255
LIC. #0004386
JOB # 252.031



PLAN VIEW

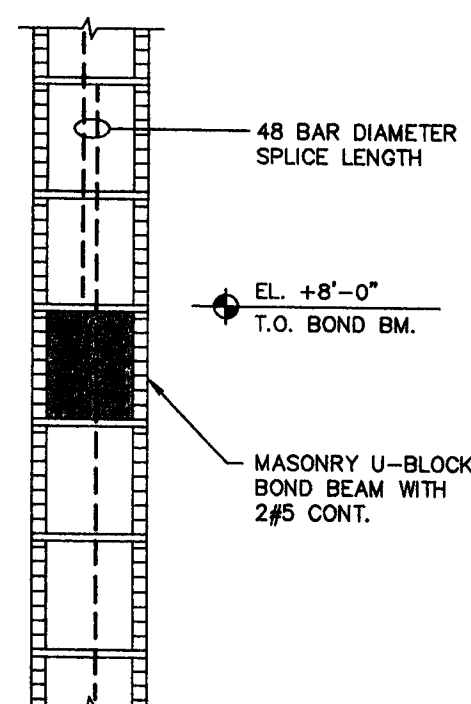
CMU WALL CONTROL JOINT (WCJ) DETAIL

NOTES:

1. SAW CUT BOND BEAMS, TIE BEAMS 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL.
2. CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0" O.C. IN WALLS WITH MORE THAN 25'-0" OF UNINTERRUPTED MASONRY. REFER TO DWGS. FOR ADDITIONAL SPECIFIED LOCATIONS AS NOTED THUS (WCJ).
3. CONTINUE ALL BOND BEAMS, TIE BEAMS REINF. THROUGH THE JOINT.

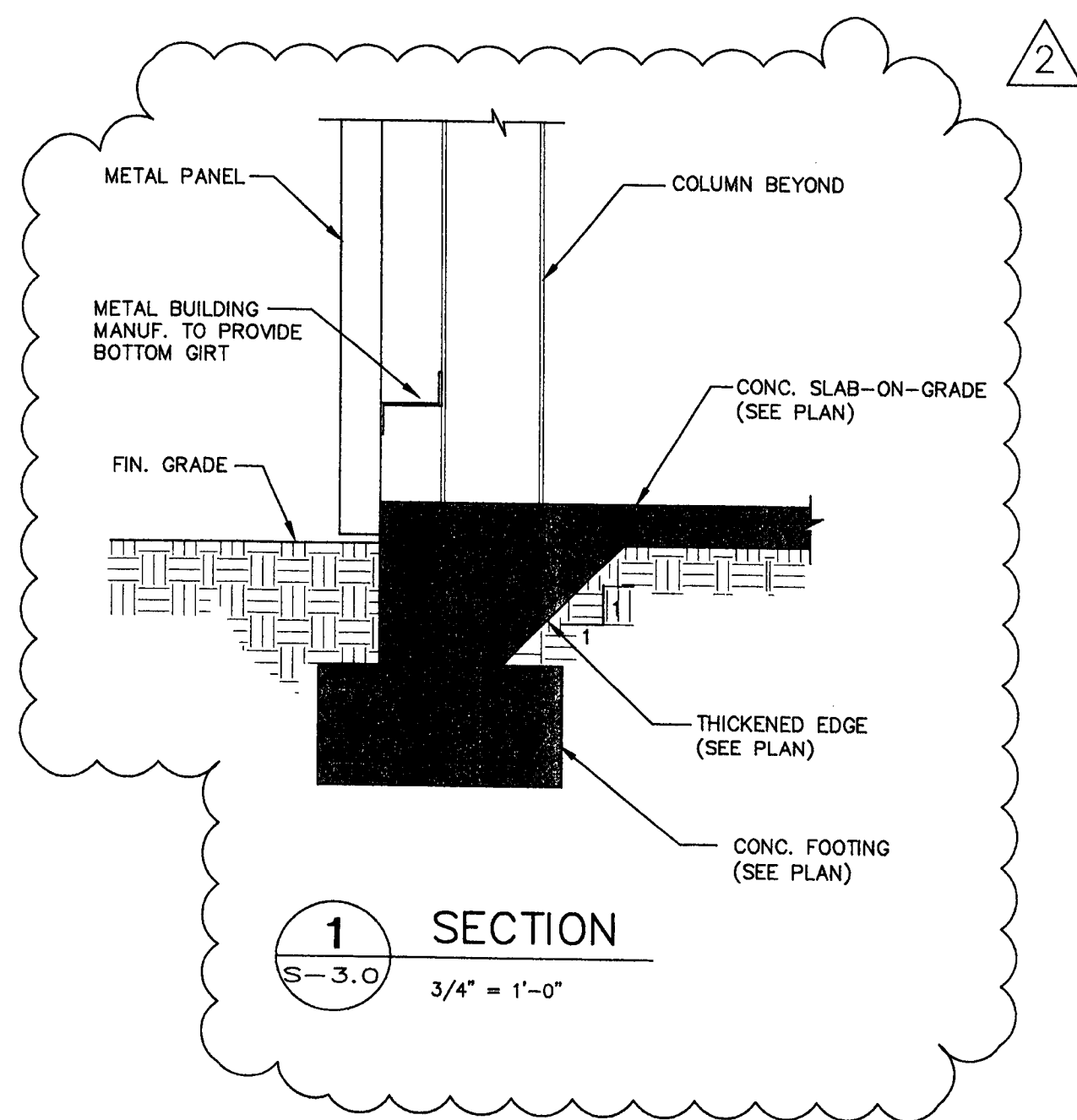
NOTES:

1. WHERE HEIGHT OF MASONRY WALL SEGMENT EXCEEDS 12'-0", INTERMEDIATE BOND BEAM IS REQUIRED.
2. BOND BEAM REINFORCEMENT MUST BE CONTINUOUS, HOOK BARS INTO REINFORCED JAMB AT WALL OPENINGS.
3. OMIT BOND BEAMS WHERE IT COINCIDES WITH OPENING HEADERS (LINTELS/BEAMS). REINFORCEMENT MUST BE CONTINUOUS AT SUCH LOCATIONS.

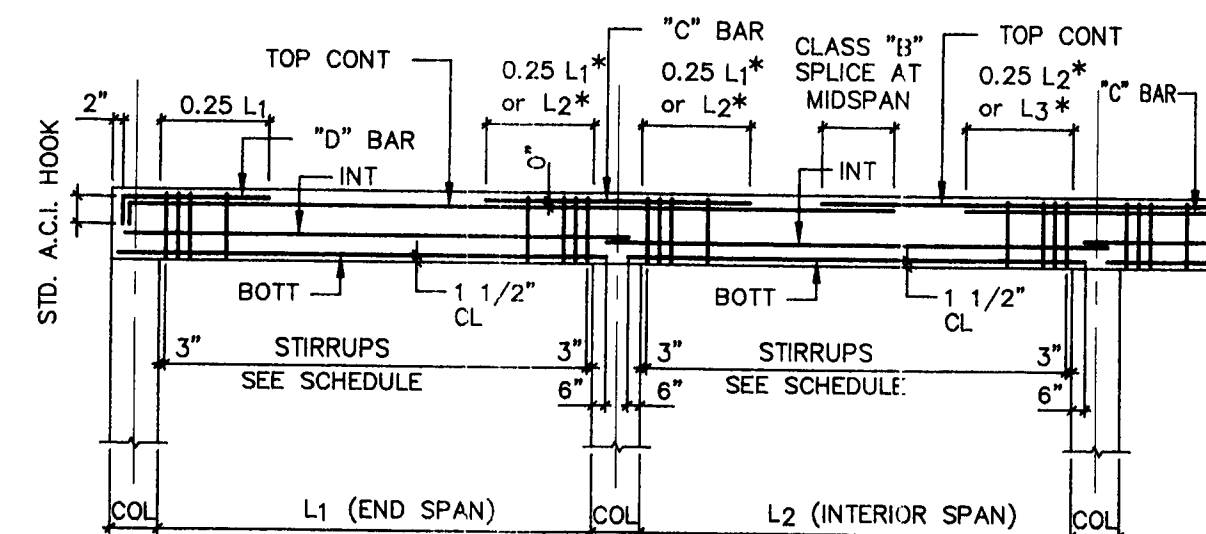


TYPICAL BOND BEAM DETAIL

N.T.S.



SECTION 1
3/4" = 1'-0"



- "C" BAR : TOP BAR AT INTERIOR SUPPORT (IN ADDITION TO TOP CONT BARS) PLACE IN SAME LAYER AS TOP CONT BARS (U.O.N.). LOCATE AT RIGHT SUPPORT OF SPAN INDICATED IN SCHEDULE.
- "D" BAR : TOP BAR AT EXTERIOR SUPPORT (IN ADDITION TO TOP CONT BARS) PLACE IN SAME LAYER AS TOP CONT BARS (U.O.N.).
- "INT" BARS : INTERMEDIATE BARS LOCATED AT A SPACING EQUAL TO THE WIDTH OF THE BEAM BUT NOT GREATER THAN 12" ABOVE BOTT BARS. IF MORE THAN ONE PAIR, PLACE IN LAYERS OF TWO.
- CLASS "B" TENSION SPLICE (3000 PSI CONCRETE)
- | # | 23" | # | 72" |
|----|-----|-----|------|
| #5 | 29" | #9 | 80" |
| #6 | 35" | #10 | 91" |
| #7 | 63" | #11 | 101" |
- * WHICHEVER IS GREATER.

NOTES:

WHEN ADJACENT BEAMS OR TIE BEAMS HAVE TOP CONT BARS OF DIFFERENT SIZE, THE TRANSITION SHOULD BE MADE AT MIDSPAN OF THE BEAM WITH SMALLER SCHEDULED BARS. USE LAP SPLICE LENGTH OF SMALLER SIZE BAR.

(2L) - INDICATES BARS PLACE IN TWO LAYERS. THE SECOND LAYER BARS MUST BE PLACED DIRECTLY UNDER BARS IN THE FIRST LAYER (IF TOP BAR) OR DIRECTLY OVER BAR IN THE FIRST LAYER (IF BOTT BAR). PROVIDE 1" CLEAR DISTANCE BETWEEN LAYERS OR ONE BAR DIAMETER, WHICHEVER IS THE GREATER DISTANCE.

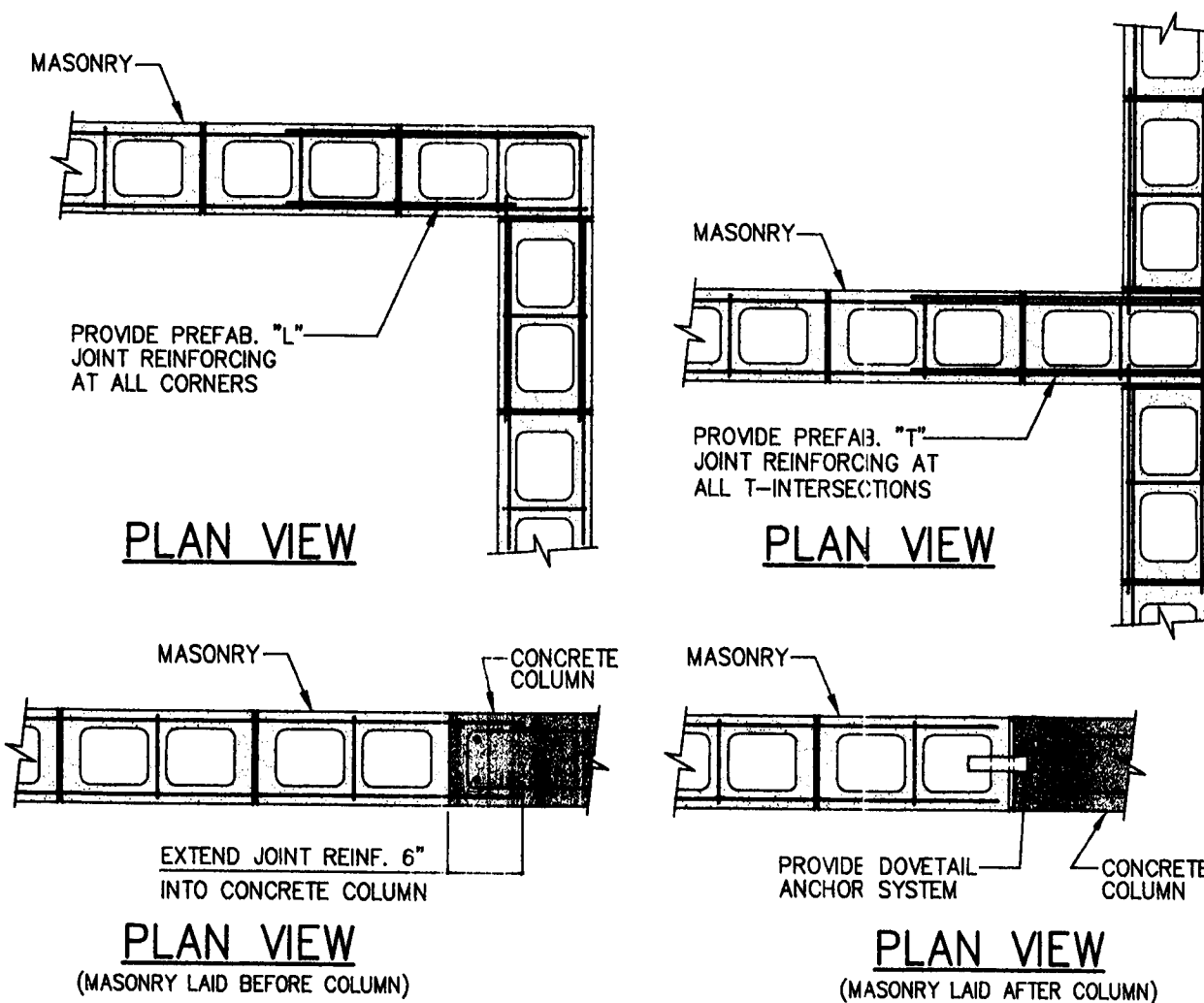
SCHEDULED BEAM SIZES : [SEE DIAGRAM A]

"B" INDICATES BEAM WIDTH DIMENSION. WHEN BEAM IS OVER A BLOCK WALL, USE ACTUAL BLOCK WIDTH (7 5/8" or 11 5/8").

"H" INDICATES BEAM DEPTH DIMENSION. LESS 1 1/2" FOR RECESS FOR BLOCK WALL DEDUCTED WHERE APPLICABLE, OR MINIMUM DEPTH IN A VARIABLE DEPTH BEAM. COORDINATE BEAM CONFIGURATION WITH ARCHITECTURAL DRAWINGS.

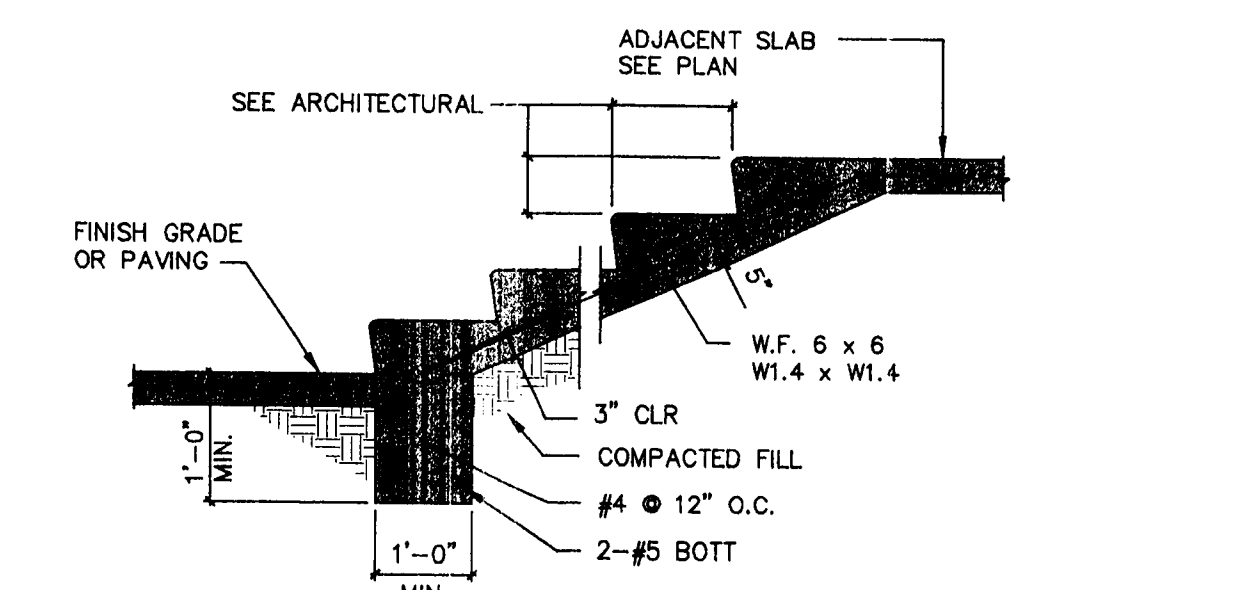
TYPICAL BEAM BAR PLACEMENT DIAGRAM

N.T.S.



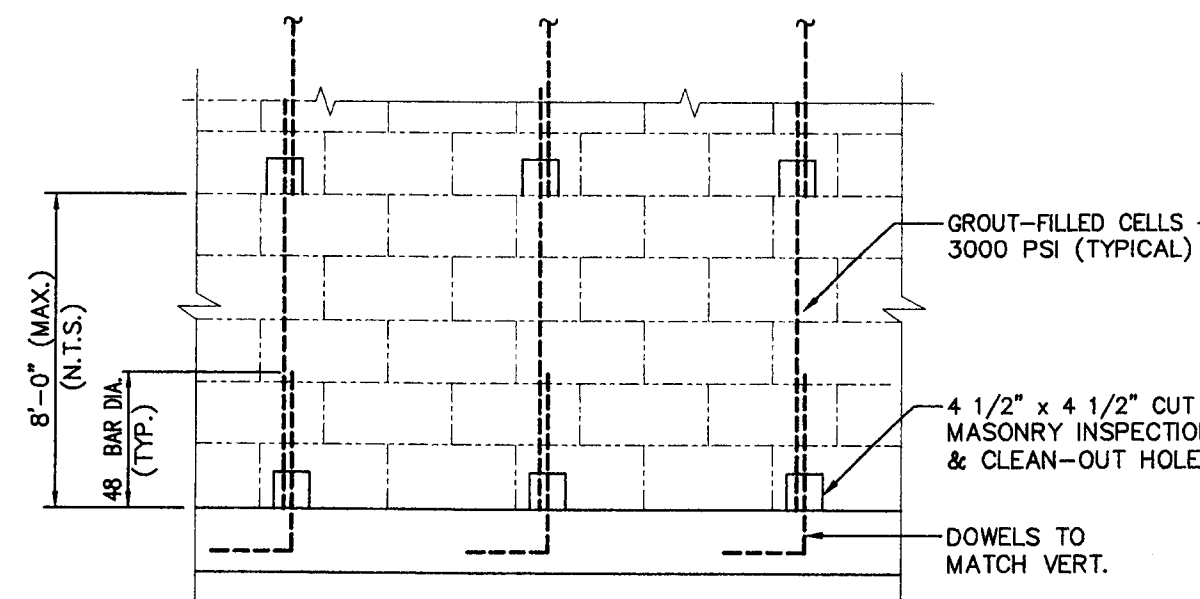
TYPICAL MASONRY DETAILS

N.T.S.

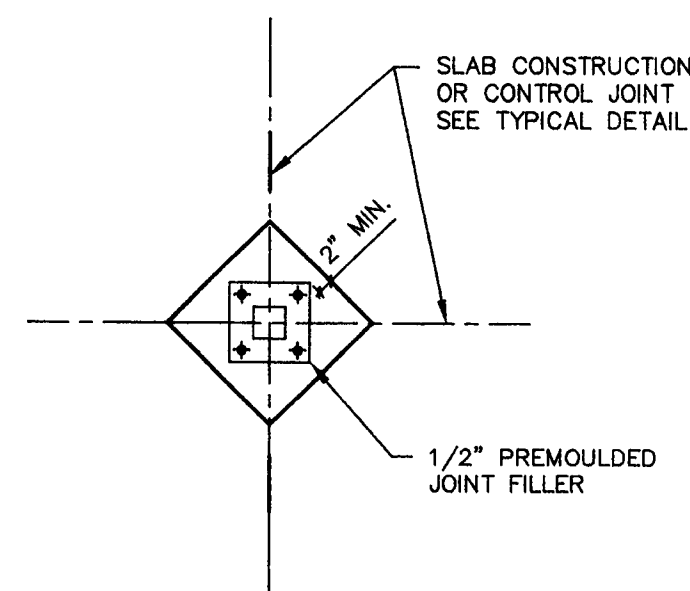


TYPICAL CONCRETE STAIR ON GRADE

N.T.S.

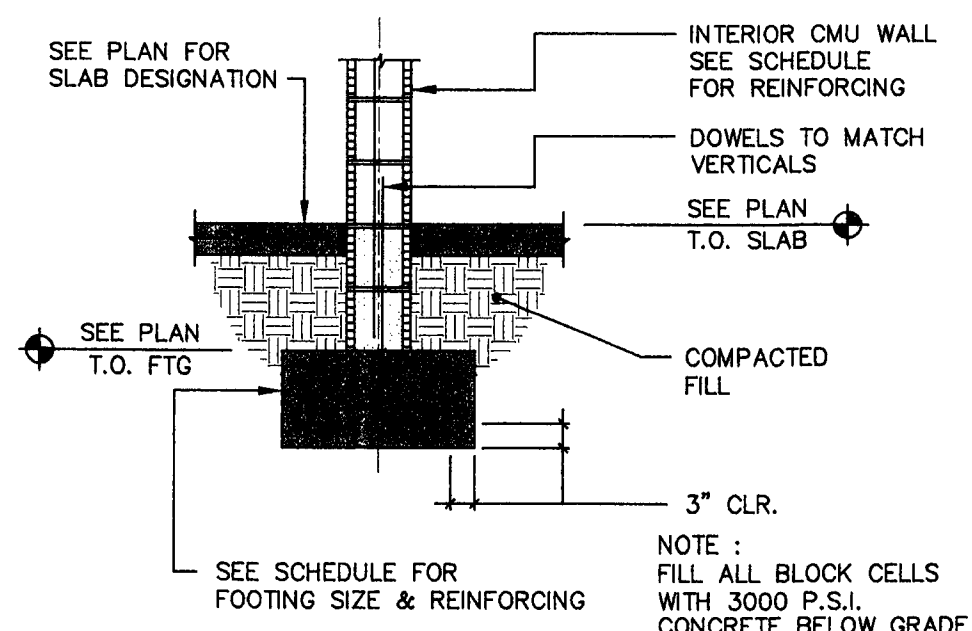


TYPICAL MASONRY FILLED CELL DETAIL



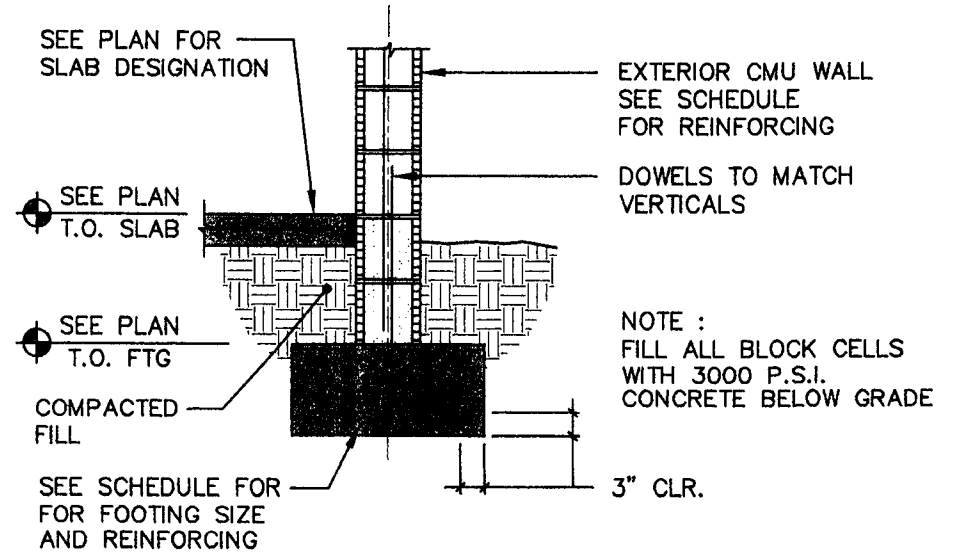
TYPICAL ISOLATION JOINTS STEEL

N.T.S.



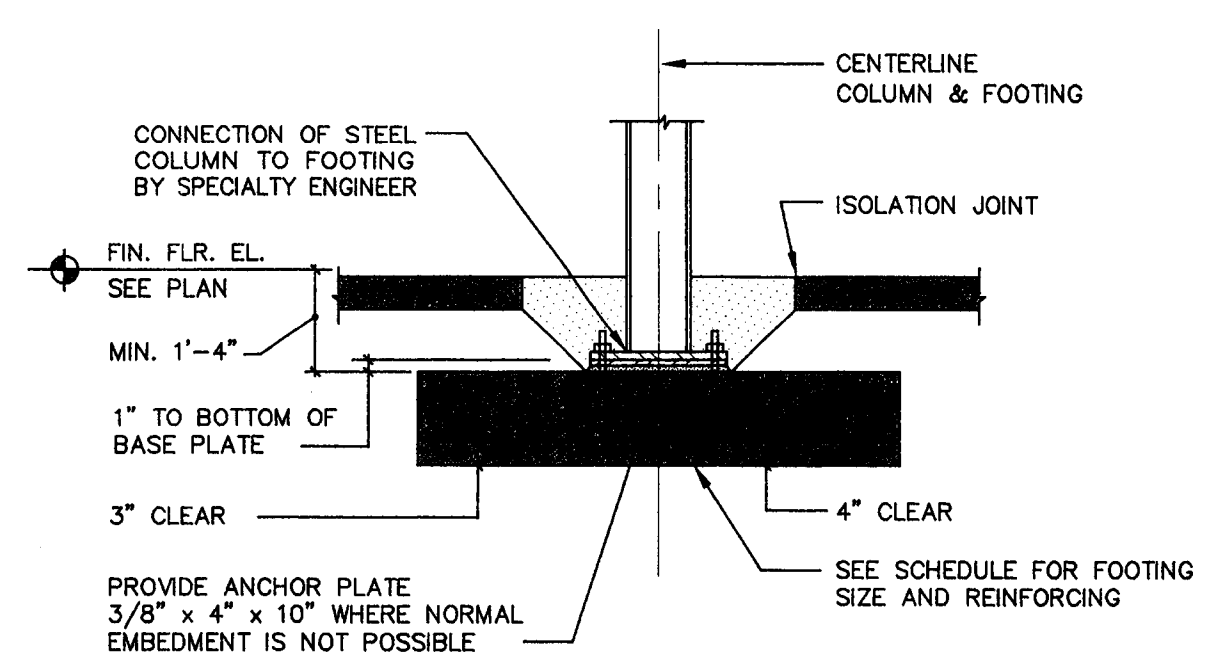
TYPICAL INTERIOR WALL FOOTING

N.T.S.



TYPICAL EXTERIOR WALL FOOTING

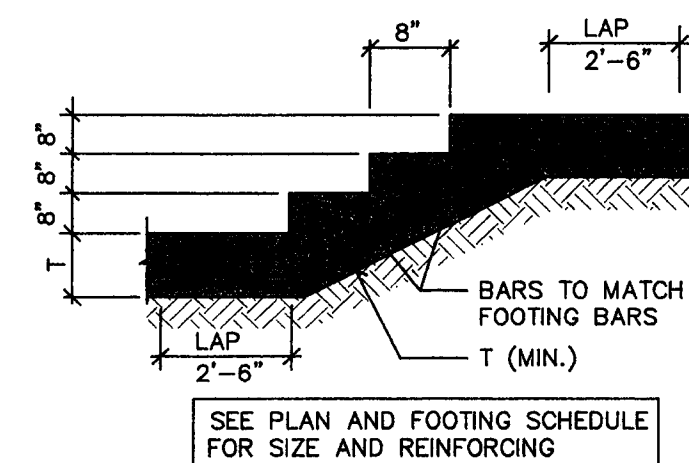
N.T.S.



TYPICAL STEEL COLUMN FOOTING

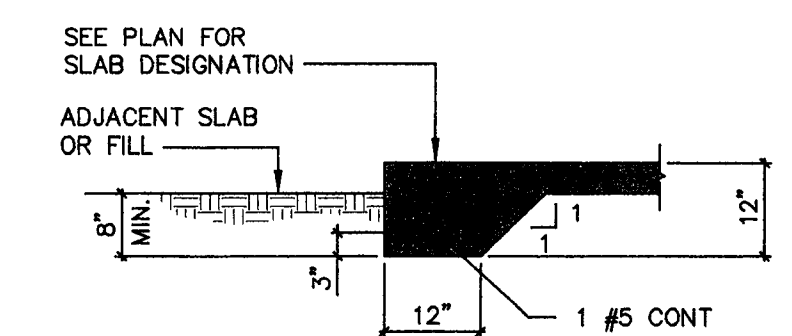
N.T.S.

x 4x4x3/8" plate where normal embedment is possible
As per John Omm 1/17/06



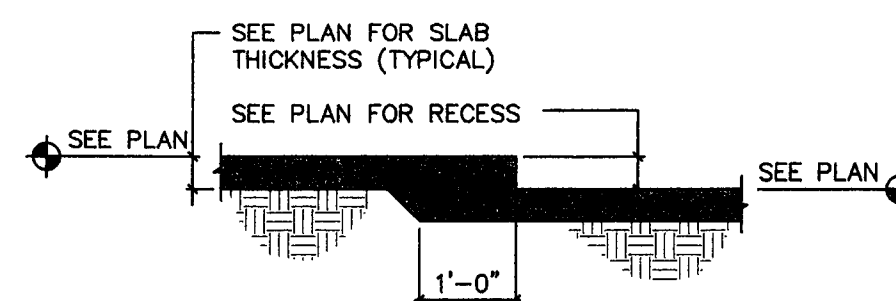
TYPICAL STEP FOOTING DETAIL

N.T.S.



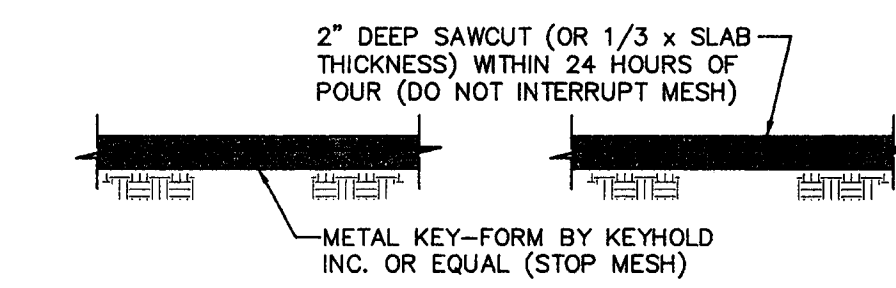
THICKENED EDGE (T.E.)

N.T.S.



TYPICAL SLAB RECESS

N.T.S.



TYPICAL SLAB-ON-GRADE

N.T.S.

NOTE: CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 400 SQ. FEET (MAXIMUM)

NOTES, SECTIONS & DETAILS

MUST COMPLY WITH ALL APPLICABLE LOCAL AND STATE CODES

REVISION

ONM

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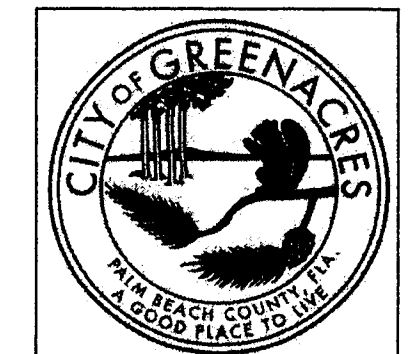
LC #0004386
JOB # 252.031

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Revisions:
Bldg. Dept. Comments/Coordination 02/07/05
Coordination 01/13/06



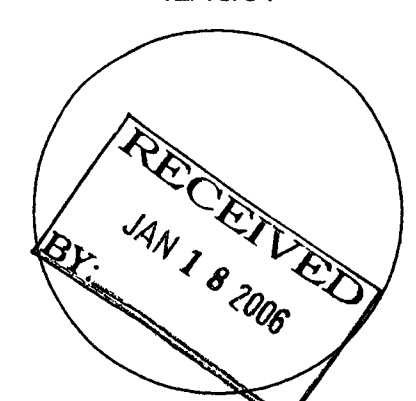
CITY OF GREENACRES - CITY HALL
5800 MELALEUCA LANE
GREENACRES, FLORIDA
CONSTRUCTION DOCUMENTS

RECEIVED by
CITY OF GREENACRES
PLANNING DEPARTMENT
JAN 17 2006

RECORD DRAWINGS
May, 2008

BUILDING PERMIT

Project Number:
02041.00
Date Issued:
12/15/04



Ian A. Nestler, AR 12428

Sheet Number:

S-3.0 PW